



Appendix 10.5 Flood Risk Assessment November 2023





53 Melville Street Edinburgh EH3 7HL +44 (0)131 226 2044 edinburgh@goodsons.com www.goodsons.co



Flood Risk Assessmer

HERDMANFLAT HOSPITAL



Aberlady Roa Haddingtc

Client:Hub South East ScotlanLtdDate:Augu: t 2023Project No:P14825Document R€P14825 - GOO- XX- XX- RP- C- 05C2



		SIGNATURE	DATE
Prepared by	Ruairidh McArthur		7 th March 2023
Checked by	Shona Tait		7 th March 2023
Version			
			DATE
Prepared by	Ella Paul		22 nd August 2023
Checked by	Shona Tait		22 nd August 2023
Version			



Contents

1.0	Introduction 1
2.0	Existing Site
2.1	I General Description
2.2	2 Site Topography
2.3	3 Ground Conditions
2.4	1 Natural Drainage Features
2.5	5 Existing Drainage Infrastructure
2.6	5 Pre-Development Discharge
3.0	Proposed Development
4.0	Flooding
4.1	1 General Requirements
4.2	2 Historical Flooding
4.3	3 Coastal Flooding7
4.4	4 Fluvial Flooding
4.5	5 Pluvial Flooding
4.6	6 Ground Water Flooding9
4.7	7 Climate Change9
5.0	Conclusions

- APPENDIX 1 Topographic Survey
- APPENDIX 2 Scottish Water Records
- APPENDIX 3 Pre-Dev. Foul & Surface Water Discharge
- APPENDIX 4 Proposed Development Plan



1.0 Introduction

Goodson Associates has been appointed by Hub Southeast Scotland Ltd to prepare a Stage 1 Flood Risk Assessment, in support of the planning application for the proposed re-development of the Herdmanflat Hospital in Haddington.

The following assessment has been completed in accordance with guidance presented within East Lothian Council's supplementary planning guidance document, Sustainable Urban Drainage Systems. It also takes cognisance of Scottish Planning Policy (SSP)1, the National Planning Framework for Scotland 4 (NPF4) and the Flood Risk Management (Scotland) Act 2009.

The aim of the Stage One FRA is to assess any potential flooding to the development site from all potential sources (coastal, fluvial [watercourse], pluvial [surface water] or groundwater). This is primarily evaluated through carrying out a desktop study of available data relating to site flood risk.

In relation to the reporting, Goodson Associates carried out the following work: -

- A site walkover with the view of potentially identifying flooding information not available from the desk study and confirming the results concluded during the desktop study.
- Review of any publicly available information on flood risk for the area.
- Liaised with the Local Authority to identify any available information on historical flooding in the area.
- Consultation of the SEPA NGR Flood Maps to obtain information on flood risk specific to the development site.
- Assessed the Flood Risk from all other sources.



2.0 Existing Site

2.1 General Description

The 5.96Ha site, which is located immediately adjacent to Aberlady Road in the town of Haddington, lies in the area administered by East Lothian Council (See Fig. 1).



Figure 1. Location plan showing the development boundary.

The site, which is broadly rectangular, is defined by the surrounding transport infrastructure. Aberlady Road, Lydgait, Hopetoun Mews and the A199 form the western, southern, eastern and northern boundaries respectively.

Residential developments, associated with previous expansions of Haddington, occupy much of the surrounding land. The exception to this is the narrow band of agricultural land that lies to the north of the site and separates it from the A1 trunk road.

An extensive band of agriculture land separates Haddington from the coastline associated with the Forth Estuary and the major settlements of Musselburgh, North Berwick and Dunbar.



2.2 Site Topography

The site generally falls from north-west to south-east, from approximately 77m above Ordnance Datum (AOD) in the northeastern corner to 57m AOD along the southern boundary. A steep embankment separates the site into two, gently sloping, plateaus (See App. 1).

The topography of the surrounding land is influenced by the Garleton Hills and tends to fall towards the River Tyne, which occupies a shallow valley approximately 1.0km to the south of the site and 0.5km to the east.

2.3 Ground Conditions

The existing ground conditions have been assessed using the data published by the British Geological Survey (BGS). From the relevant borehole and trial pit logs the following typical sequence of strata was identified:

- 1 Topsoil –varying in thickness, but generally 0.3m.
- 2. Glacial Till (Cohesive) Firm to stiff, sandy, gravelly, silty clay, approximately 4.0m deep.
- 3. Bedrock Rhyolite.

The presence of the notionally impermeable clay layer makes the use of traditional infiltration techniques inappropriate for the site.

2.4 Natural Drainage Features

An analysis of the available Ordnance Survey data shows that the site naturally forms part of the catchment of the River Tyne (See Fig. 2).



Figure 2. Map extract showing the water courses within the vicinity of the site.

Rising in the Moorfoot Hills, near Tynehead in Midlothian, the watercourse follows a north-easterly path to Tyninghame, where it discharges to the North Sea. Although almost 1.0km from the southern boundary of the site, the watercourse meanders to the north as it passes through the eastern area of the town and approaches to within 500m of the eastern boundary.

The Letham Burn and the St Laurence House Burn are tributaries of the River Tyne that lie to the west of the proposed development site.

The Harperdean Burn rises approximately 0.8km to the north of the site, on the southern slopes of Barney Hill. Flowing south towards the River Tyne, the watercourse enters a culvert to the north of the A1 trunk road. The 900mm diameter culvert discharges to the River Tyne at Bothwell Bank. The available record drawings show that a branch of the Harperdean culvert flows south, along Victoria Park. The 300mm diameter vitrified clay culvert turns to the east at the junction with Newton Port. Increasing in diameter to 450mm as it passes Tenterfield House, the branch meets the main line at the junction between Tenterfield Drive and Hardgate (See App. 2).

The River Tyne is currently considered to be of poor water quality, however in their River Basin Management Plan SEPA have set the goal of obtaining moderate water quality by 2027. The long-term aim is to achieve good water quality.



2.5 Existing Drainage Infrastructure

The Scottish Water records show that developments adjacent to the site are served by a mixture of publicly owned combined sewers, surface water sewers and foul sewers.

A 450mm vitrified clay, combined sewer, serving the residential area to the west of the site, is present within the western section of Lydgait (See App. 2). Turning to the south at Lydgait Court the sewer follows the line of Market Court to Fortune Avenue, where it resumes it's easterly path towards Haddington Wastewater Treatment works –located 2.0km to the east of the site, adjacent to Amisfield Walled Garden.

A smaller combined sewer, that originates in the eastern portion of Lydgait, serves Hopetoun Mews. Flowing east towards Victoria Park, the sewer turns to the south and discharges to the trunk sewer at Newton Port.

The available records suggest that combined sewer overflows (CSO), that discharges to the River Tyne, are present adjacent to the Pure Malt distillery.

Small diameter foul and surface water sewers serve the properties associated with Lydgait Gardens and Market Court.

2.6 Pre-Development Discharge

The site is currently occupied by Herdmanflat Hospital, which closed in 2020 when services were transferred to the East Lothian Community Hospital. Therefore, it has been assumed that the site discharges to the adjacent sewerage network (Table 1). Consequently, the surface water discharge has been estimated using the recommendations of BS EN 752 (See App. 3).

The pre-development foul water discharge associated with the hospital has been determined using the recommendations of British Water's Flows and Loads 4.

Table 1. Existing discharge to the sewerage system.

	Surface	Foul Water	Tota I
	Water	(I/s)	(I/s)
	(l/s)		
Characteristic Discharge to combined	-	0.47	0.47
Sewer			
Peak Discharge to Combined Se	132.7	2.82	135.52



3.0 Proposed Development

The proposed development is to consist of the construction of up to 96 sheltered housing units, the associated car parking and access roads. The area of land required for each element has been estimated using the preliminary site layout (Table 2 and App. 4).

Table 2. Land requirements

Usage	Area (ha)	Total Area (ha	Surfacing Characteristics
Residential Roc Roads, Footpaths & Residential Parki	0.578	. .	lm perm eable Im perm eable
Total Impermeable Ar		1796	
Landscaping/Undeveloped A	4.904	4.904	Perm eable
Total Area Developme		6.700	

4.0 Flooding

4.1 General Requirements

The planning process requires that it be demonstrated that the site can be developed whilst:

- 1 Maintaining an acceptable risk of flooding and;
- 2. Ensuring that the flood risk is not increased elsewhere.

The risk framework set out in Scotland's National Planning Framework 4 (NPF4) and SEPA's complimentary document, Flood Risk and Land Use Vulnerability Guidance, uses the concept of "land-use vulnerability" to define acceptable flood risks.

The appropriate risk classifications have been determined using the Flood Risk Matrix, in conjunction with the land-use vulnerability classification. In accordance with NPF4, the relevant climate change allowance should also be considered.



Table 3. Land-use classification and acceptable flood risk

Land Use	Vulnerability	Little or No Risk	Low to Medium	Medium to High
	Classification		Risk	Risk
		(<1100C)	(1:100C to 1:200)	(>1200)
Dwellings	Highly	No Constraints	Generally	Generally
	Vulnerabl		Suitable	not suitabl ¹

Notes

1. Generally not suitable for development unless one of the following apply:

a. Redevelopment of an existing building, including changes of use to an equal or less vulnerable use to the existing use.

b. Redevelopment of a previously developed site where it involves the demolition of existing buildings and/or erection of additional buildings within a development site, and the proposed land use is equal or less vulnerable than the existing land use.

c. Where the principle of development on the site has been established in an up-to-date, adopted development plan or the National Planning Framework and flood risk issues were given due consideration as part of the plan preparation process and our assessment of risk has not changed in the interim.

d. The site is protected by a flood protection scheme of the appropriate standard that is already in existence and maintained, is under construction, or is planned for in a current flood risk manage

It should be noted that the flood risk classification does not relate solely to flooding from natural watercourses, but rather to all potential sources of flooding relevant to the development site –each of which is discussed below.

4.2 Historical Flooding

From the Forth Estuary, Flood Risk Management Strategy, there is no indication of any prior flood events on the proposed development site.

4.3 Coastal Flooding

The site lies 7.5km from the shoreline of the Forth Estuary, therefore it is not directly affected by the daily fluctuations of the tide.

A study of the effect of extreme surge tides on the East Coast of Scotland, commissioned by The Scottish Environment Protection Agency (SEPA), suggests that the 1 in 200-year sea level within the Forth Estuary can be expected to be 4.2 m Above Ordnance Datum (AOD). Supplementing this with the analysis contained within the UK Climate Change Projections 2009 (UKCP09), the maximum tidal level that can reasonably be expected, when adjusted to account for climate change, is 4.69m – considerably lower than the likely lowest boundary level of approximately 57.0 m.

The risk of tidal flooding is, therefore, considered to be negligible.



4.4 Fluvial Flooding

In accordance with East Lothian Council's Flood Prevention Guidelines it must be demonstrated that the development does not have a detrimental impact on the 1:200 year return period floodplain.

The SEPA Indicative River and Coastal Flooding Map, which is based on a probability of flooding of 0.5%, or the 1 in 200-year return period, indicates that the River Tyne is susceptible to flooding along its length.

Superimposing the flood extents on to the available Ordnance Survey level data suggests a peak flood water level of approximately 45m at the closest point to the site.

The lowest boundary level of the proposed development site is 57m AOD, which provides a significant factor of safety against flooding from the nearest watercourse.

The Haddington Flood Study, commissioned and published by East Lothian Council, also suggests that the proposed development site lies outwith the 1:200-year flood plain.

The risk posed by fluvial flooding is therefore considered to be negligible.

4.5 Pluvial Flooding

Overland flooding may be generated by three sources:

- 1 Over-topping of the onsite drainage system.
- 2. Flash run-off from adjacent ground or the public highway –typically caused by blocked drainage inlets or extreme rainfall events.
- 3. Over-topping of the adjacent publicly owned sewerage system or other infrastructure.

The drainage system constructed to serve the site, is to be designed in accordance with East Lothian Council's requirements, therefore it will:

- 1 Provide sufficient attenuation to accommodate a 1:200-year (plus 40% climate change) return period storm event.
- 2. Limit the peak discharge rate from the site.

On this basis the internal drainage system will not expose the development site to the risk of pluvial flooding within the relevant design return period. In the event of the system malfunctioning, due to blockage, the road corridors would act to channel water away from the proposed buildings.

The site occupies an area at the base of the southern slope of Barney Hill. Consequently, a long, steep slope extends from the northern boundary to the summit of the circular hill. Although the existing vegetation and the topsoil act to intercept overland flow during short return period rainfall events, there remains the potential for overland flow during more intense storms.



Any overland flow generated by the south-western quadrant of Barney Hill is currently intercepted by the highway drainage system serving the A1 trunk road and the A199, which act to convey water towards the lower ground to the east.

The site is therefore considered to be at low risk of pluvial flooding.

4.6 Ground Water Flooding

The preliminary assessment of the ground conditions suggests that any groundwater encountered will be either:

- 1. Perched within the cohesive sub-soils.
- 2. Flowing through fractures in the underlying bedrock.

As the proposed earthworks are unlikely to lead the complete removal of the cohesive deposits, this impermeable layer will continue to act as an effective barrier to the upward migration of ground water.

While the impermeably surfaced areas will prevent percolation of rainwater into the underlying soils, the soft-landscaping will allow some percolation to occur –although plant-life will reduce this through interception and absorption.

As the proposed development is to retain the basic topography of the site, any groundwater flow will continue to make its way, towards the lower ground to the south-east of the site.

The risk of flooding from this source is therefore considered to be low.

4.7 Climate Change

Although the effects of climate change will vary at a regional level, throughout the UK more frequent short-duration, high intensity rainfall and more frequent periods of long-duration rainfall are to be expected.

In compliance with East Lothian Council Flood Risk Requirements the development will be designed to ensure that it is not at risk of flooding from the relevant return period event, with a 40% uplift to account for climate change.

The proposed surface water drainage network shall be designed to contain flood flows generated up to and including the 1 in 200 years plus climate change storm events within the site, without damage to buildings, essential services, or neighbouring developments.

5.0 Conclusions

The type 1 flood risk assessment presented above confirms that the site is at low risk of flooding from coastal, fluvial, pluvial and groundwater sources. The site can therefore be deemed suitable for the type of development proposed, without the need for a more detailed, type 2 or type 3 study.

APPENDIX 1 TOPOGRAPHIC SURVEY



APPENDIX 2 SCOTTISH WATER RECORDS





APPENDIX 3 PRE-DEV. FOUL & SURFACE WATER DISCHARGE

Goodson		Contract Herdmanflat Ho	ospital, Haddington
Aggagiatag	Consulting Civil and Structural Engineers	Prepared by R McA	^{Date} Mar 23
Associates		P14825	^{Sheet No} FW 1 ^{Rev}
Title Pre-Development FW Di	ischarge		
Use the po the recom Flows and	opulation method to estimat mendations of Sewers for So Loads 4	e the pre-development foul di cotland (4th Edition), BS EN 75	scharge, in accordance with 2 and British Water
<u>Developm</u>	ent Details and Characterist	tic Flow Rate	
The site wa and geriati	as previously occupied by a l ric care. At the time of closu	Herdmanflat Hospital, which p re:	rovided mental health services
Patient	s - 100		
Staff - 6	55		
Flow Chara	acteristics	Herdmanflat Hospital, Haddington Prepared by R McA Date Mar 23 Ref No P14825 Sheet No FW 1 Rev e n method to estimate the pre-development foul discharge, in accordance with ions of Sewers for Scotland (4th Edition), BS EN 752 and British Water alls and Characteristic Flow Rate ously occupied by a Herdmanflat Hospital, which provided mental health services At the time of closure: r, Flows and Loads 4: //hd per day (Residential old people / nursing) ber day charge (for Assessment of Impact upon Scotlish Water Waste Water Treatment Works) f 100 x 350 for S x 90 for S x 90 for X = 5850 for X = 5850 for X = 40850 I/day	
From Britis	sh Water, Flows and Loads 4	:	
Patient	s - 350 l/hd per day	(Residential old people / nu	rsing)
Staff - S	90 l/hd per day		
<u>Characteri</u>	stic Discharge (For Assessment of	Impact upon Scottish Water Waste Water	Treatment Works)
Total Disch	narge -		
Patient	s = 100 x 350		= 35000
Staff	= 65 x 90		= 5850
Total			= 40850 I/day
Q _{WW Char.}	= 40850 / (24 x60 x 60) = 0.47 l/s		

Castan	•	Contract Herdmanflat Hospital, Haddington			
Goodson	Consulting Civil and Structural Engineers	Prepared by R McA	Date Mar 23		
Associates		Ref No P14825	Sheet No FW 2 Rev		
Title Pre-Development FW Di	scharge				
Peak Disch	IATGE (For Assessment of Impact upor	n Scottish Water Waste Water network)			
A peaking	factor of 6 is typcally used to	Consulting Civil and Structural Engineers Prepared by R McA Date Mar 23 Ref No P14825 Sheet No FW 2 Rev arge () for Assessment of Impact upon Scattish Water Weste Water network) or of 6 is typically used to estimate the design flow, therefore: = 0.47 x 6 = 2.82 l/s			
Q _{WW Peak}	= 0.47 x 6				
	= 2.82 l/s				

Goodson			Contract Herdmanflat Hospital, Haddington			
Goods	son •	Consulting Civil and Structural Engineers	Prepared by R McA	Date Mar 23		
Assoc	lates	,	Ref No P14825	Sheet No PASD 1 Rev	,	
Title Pre-Dev	elopment SW Di	scharge				
Fig. NA 2	Consider the therefore of recommend Rainfall Inter- Consider the Therefore: Consider R Design Rain Therefore: Rainfall Inter Impermead Impermead Peak Disch Q = 2.78 = 2.78 = 132	he existing drainage system determine the pre-attenua idations. tensity hat Ponding can be tolerate isk Category 1 infall Frequency = 1 in 1 yea rensity = 36.0 mm/hr ble Area ble Area = 1.362 ha harge 3 x C _v x C _r x r x A 3 x 0.75 x 1.3 x 36.0 x 1.36 .7 I/s	n to have been designed ted surface water discha ed during heavy rainfall a ar(s)	I in accordance with BS EN 752, arge in accordance with its		

Goode	ion	Consulting Civil and Structural Engineers	Contract Herdmanflat Hospital, Haddington			
Goods			Prepared by	′ R McA	Date Mar 23	
Assoc	lates		Ref No	P14825	Sheet No PASD 1 Rev	
Title Pre-Deve	elopment SW D	ischarge				
Fig. NA 2	Consider therefore recommendation Rainfall In Consider the Therefore Consider F Design Rain Therefore Rainfall In Impermean Peak Disci Q = 2.7° = 2.7° = 175°	he existing drainage system determine the pre-attenual ndations. tensity hat Ponding can be tolerat sisk Category 1 infall Frequency = 1 in 1 yea infall Frequency = 1 in 1 yea is tensity = 36.0 mm/hr able Area ble Area ble Area = 1.796 ha harge 8 x C _v x C _r x r x A 8 x 0.75 x 1.3 x 36.0 x 1.80 5.6 l/s	n to have bee ited surface w	en designed in acc vater discharge in	ordance with BS EN 752, accordance with its r a few minutes afterwards.	

APPENDIX 4 PROPOSED DEVELOPMENT PLAN



KEY 1:1000

-	RED LINE BOUNDARY
)	LANDSCAPE PRINCIPLES Orchard and community garden
)	Formal terrace
)	Primary Area of community open space
)	Recreational space with pétanque pitches
\mathcal{D}	Private terrace - 3m strip
\mathbf{D}	Semi-private spaces
)	Green corridor - succession planting and paths Woodland - succession planting, paths and incidental play
\mathcal{D}	Protected tree line avenues with succession planting

- 9 Wet meadow creation linked to SUDS
- 10 Private gardens for complex care use
- Existing boundary wall retained
- Active travel routes connecting site to wider area 3m wide

						÷
				-		_
01	366.09.23	First Issue		AR	bok .	Tild
\$8	Date	Description		Dm	Chk	Арр
						-
	Scale 1:1	000		1		
3	0	10 20 3	30 40 50n	n		1
1	U.	Bristol Edinburgh Glasgow London	landus	e.co.i	ık	
Pr H	oject erdma	anflat Hospita	d			
Ch	ent ub So	uth Fast Scot	and			
		Seals (541	Chabie			
4	100014	1.1000	Diamaina			
1	1288	1:1000	Planning			
Dr	awing Titl	e				
L	andsc	ape General A	rrangement			
Ш	ustrati	ve				
	ence a difference in the	2000 A			_	_
Dr	awing Nr				Issu	Ð
Dr 1	awing Nr 1288 I	D. PL N. 101			issu D	e 01